

Zeitschrift: IABSE reports = Rapports AIPC = IVBH Berichte
Band: 60 (1990)

Artikel: Behaviour of partially concrete-filled steel columns
Autor: Fujiwara, M. / Minosaku, K. / Takahashi, N.
DOI: <https://doi.org/10.5169/seals-46442>

Nutzungsbedingungen

Die ETH-Bibliothek ist die Anbieterin der digitalisierten Zeitschriften auf E-Periodica. Sie besitzt keine Urheberrechte an den Zeitschriften und ist nicht verantwortlich für deren Inhalte. Die Rechte liegen in der Regel bei den Herausgebern beziehungsweise den externen Rechteinhabern. Das Veröffentlichen von Bildern in Print- und Online-Publikationen sowie auf Social Media-Kanälen oder Webseiten ist nur mit vorheriger Genehmigung der Rechteinhaber erlaubt. [Mehr erfahren](#)

Conditions d'utilisation

L'ETH Library est le fournisseur des revues numérisées. Elle ne détient aucun droit d'auteur sur les revues et n'est pas responsable de leur contenu. En règle générale, les droits sont détenus par les éditeurs ou les détenteurs de droits externes. La reproduction d'images dans des publications imprimées ou en ligne ainsi que sur des canaux de médias sociaux ou des sites web n'est autorisée qu'avec l'accord préalable des détenteurs des droits. [En savoir plus](#)

Terms of use

The ETH Library is the provider of the digitised journals. It does not own any copyrights to the journals and is not responsible for their content. The rights usually lie with the publishers or the external rights holders. Publishing images in print and online publications, as well as on social media channels or websites, is only permitted with the prior consent of the rights holders. [Find out more](#)

Download PDF: 01.04.2026

ETH-Bibliothek Zürich, E-Periodica, <https://www.e-periodica.ch>

Behavior of Partially Concrete-Filled Steel Columns

Comportement des colonnes d'acier partiellement remplies de béton

Verhalten von teilweise mit Beton gefüllten Stützen

M. FUJIWARA

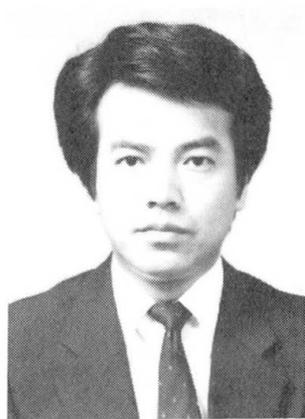
Head, Bridge Div.
PWRI
Tsukuba, Japan



Minoru Fujiwara, born 1942, received M.Eng. at Nagoya University in 1967. He has been engaged in bridge engineering works and road administration since 1967.

K. MINOSAKU

Res. Eng.
PWRI
Tsukuba, Japan



Koichi Minosaku, born 1956, received M.Eng. at Tottori University in 1982. He has been engaged in bridge engineering works since 1982.

N. TAKAHASHI

Assist. of Gen. Mgr
Kansai Airport Corp.
Osaka, Japan



Noboru Takahashi, born 1945, received B.S. at Tokyo Institute of Technology. He has been engaged in bridge engineering in KIACL and railway construction since 1968.

H. KAWAGUCHI

Head, 1st Design
Div.
Kansai Airport Corp.
Osaka, Japan



Hiroshi Kawaguchi, born 1948, received B.S. at Nagoya University in 1971. He has been engaged in bridge engineering in KIACL and railway construction since 1971.

SUMMARY

Concrete is filled inside the lower columns of two-storied rigid frame piers of Kansai International Airport Bridge located within Osaka Bay. This paper presents the results of a bi-directional loading tests performed to understand the behavior of partially-concrete-filled steel columns.

RÉSUMÉ

Le béton est coulé dans la partie inférieure des piles d'un pont à cadre rigide à 2 niveaux. Cet ouvrage dessert l'aéroport international de Kansai situé dans la baie de Osaka. Ce document présente les résultats d'une analyse de chargement bidirectionnel permettant de comprendre le comportement des piles d'acier partiellement remplies de béton.

ZUSAMMENFASSUNG

In die unteren Stützen eines zweistöckigen Rahmens wurde Beton eingefüllt. Die Brücke befindet sich in der Nähe des Kansais-Flughafens. Dieser Bericht stellt die Resultate einer Übergewichtsprüfung vor, um das Verhalten der mit Beton gefüllten Stützen zu erklären.



1. Introduction

Kansai International Airport(KIA) Bridge is now under construction by KIA Co. Ltd. to be a 3.75km dual purpose (road and railway) route connecting the airport island with Osaka area. The superstructure of the KIA Bridge has three main sections. The type of superstructure over open water is 3-span continuous steel truss with equal span length of 150m. The type of superstructure for each approach ends is 3-span continuous steel box girder with equal span length of 109m. The type of the KIA Bridge pier is two storied rigid frame steel structure as shown in Fig. 1. The lower column of the frame was designed to provide high ductility and load carrying capacity against the seismic force by filling concrete partially. It was already verified that wholly-concrete-filled steel columns have high ductility and load carrying capacity'. However, no study on the behavior of partially-concrete-filled steel columns is reported yet. For this reason, Public Works Research Institute(PWRI), Ministry of Construction, carried out two directional loading test for model specimens of partially-concrete-filled steel columns in collaboration with KIA Co. Ltd. This paper presents the outline of the results of this loading test.

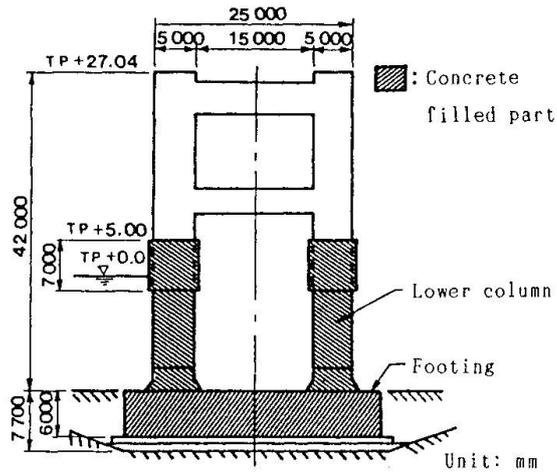


Fig. 1 Shape of Kansai International Airport Bridge Pier

2. Test method

2.1 Specimens

Three specimens which are almost 1/5 scale model of the lower column of the KIA Bridge pier, were fabricated using steel plates of grade SM41. The concrete was filled by up to half depth from the bottom for every specimen. They have same height of 350cm and same cross section of 90×90cm. Fig. 2 shows their dimensions. Lateral diaphragms were provided for every specimen in the same manner as those of the KIA Bridge pier in order to expect the composite action between the concrete and the steel column. A closed type lateral diaphragm was

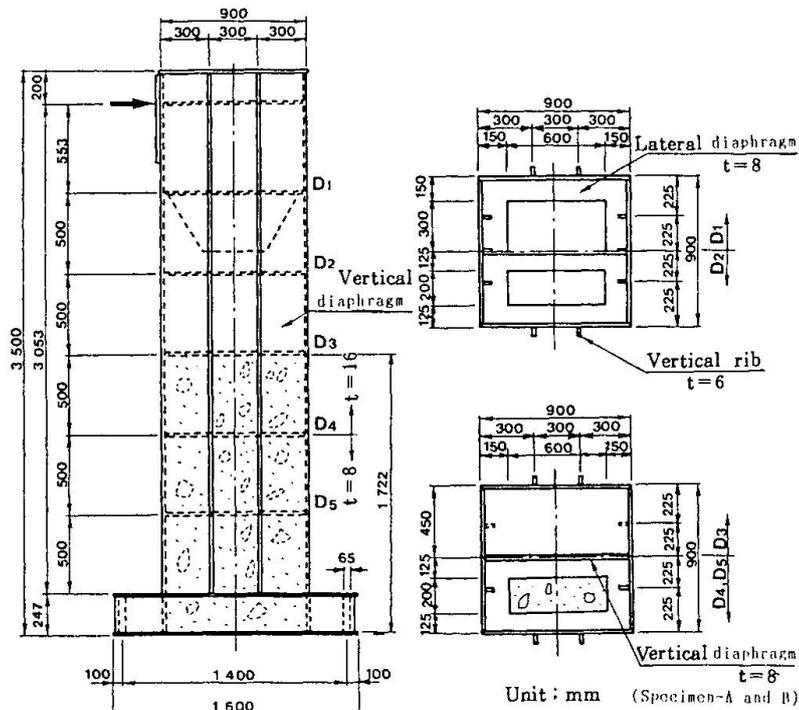


Fig. 2 Configuration of specimens

Table 1 Strength of steel (SM41)

Thickness(mm)	8	16
Yield point(kgf/cm ²)	3,540	2,950
Tensile strength(kgf/cm ²)	4,770	4,480

Table 2 Properties of concrete
(concrete cylinder: 10×20cm)

Specimen	A	B	C
Compression Strength(kgf/cm ²)	267	260	253
Tensile Strength (kgf/cm ²)	26.1	21.6	23.4
Young's modulus(×10 ⁵ kgf/cm ²)	2.27	2.17	2.14

placed at the boundary between concrete-filled part and non-concrete-filled part to impart the vertical load to the filled concrete, while hollow type lateral diaphragms were used to other parts. A vertical diaphragm was placed in Specimen-A and Specimen-B to expect the smooth stress flow at transition zone of concrete-filled part and non-concrete-filled part. But Specimen-C does not have a vertical diaphragm. The concrete was so mixed that the compression strength(240kgf/cm²) of the concrete can be obtained. Table 1 and Table 2 show material properties of the concrete and the steel plate obtained from the material tests, respectively.

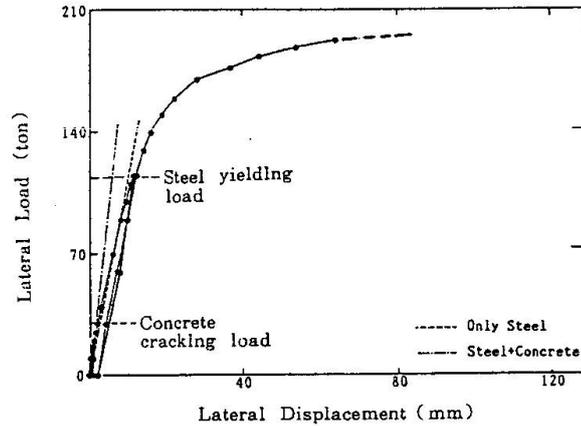
2.2 Loading method

The lower edge of specimens was fixed to the bed of the loading machine with bolts. The lateral load was statically applied to the point of 20cm lower from the upper edge under constant vertical load of 100ton for Specimen-A and Specimen-C, 500ton for Specimen-B. The vertical stress under the vertical load of 100ton is almost same as that in the KIA Bridge pier. In the loading test, the vertical and lateral displacement of specimens as well as strains in the steel plate of the steel column and the vertical diaphragm were measured. The strains inside the filled concrete was also measured by mould gages embeded inside the steel column.

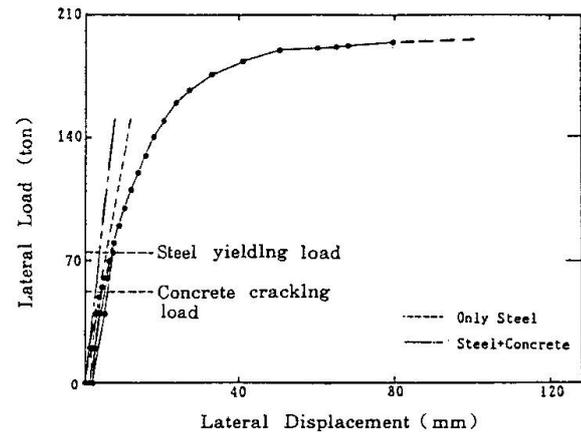
3. Test results

3.1 Load-displacement relationship

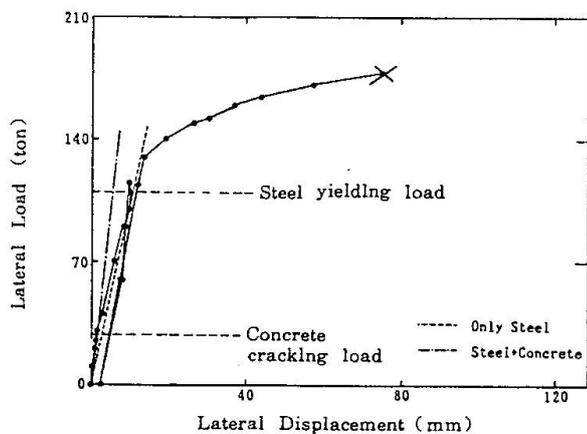
Fig. 3 (a)~(c) show the relationship between lateral load and lateral



(a) Specimen-A (Vertical Load: 100ton)



(b) Specimen-B (Vertical load: 500ton)



(c) Specimen-C (Vertical load: 100ton)

Fig. 3 Load-displacement relationship



displacement at the loading point for every specimen. In every specimen, a bending crack of the filled concrete firstly might have occurred in the tension side of the lower edge according to the strain measured by mould gages, and subsequently the yielding and small local buckling of the steel column occurred in the compression side of the lower edge. Maximum lateral load for every specimen was observed after these phenomena with high ductility. Two lines in Fig. 3 (a)~(c) which indicate the theoretical load-displacement relationship are based on the elastic theory. A dashed line is based on the assumption of the flexural rigidity of steel column only, while a dot-dashed line is based on the assumption of the combined flexural rigidity of the steel column and the filled concrete.

From these figures, it is noticed that the flexural rigidity of the filled concrete leads to the increase of the flexural rigidity of specimens particularly before the occurrence of the bending crack of the filled concrete, because measured displacements of every specimens increase almost along dot-dashed lines. Although the lateral loading for Specimen-A and Specimen-B were performed taking into account the capacity of the loading machine until their lateral displacements reach respectively about 5.3 and 11.3 times the yielding one, their failure points were not observed. On the other hand, Specimen-C lost its resistance due to the crack of the steel column in the tension side of the lower edge when the ratio of the lateral displacement to the yielding one was about 6.3.

3.2 Load carrying capacity

Table 3 lists the concrete cracking load and the steel yielding load at lower edge as well as the maximum load. Theoretical values are based on the the assumption of the strain distributions in cross section as shown in Table 4. The theoretical concrete cracking load (P_{cc}) was based on the assumption that the crack occurs when the stress in the filled concrete reaches the splitting tensile strength. The value P_{tys} expresses theoretical steel yielding load in the case that only steel column resists the lateral load, while the value P_{tysc} indicates theoretical steel yielding load in the case that both the steel column and the filled concrete resist the lateral load. The value P_{uh} expresses theoretical maximum load based on the calculation method²⁾ proposed for wholly-concrete-filled steel columns, while the value P_{up} indicates theoretical maximum load corresponding to the full plastic resisting bending moment in which the effect of the strain hardening of the steel plates is not considered.

Test results for every specimen show that the measured concrete cracking load (P_{cm}) agrees well with the theoretical value, but

Table 3 Test results (concrete cracking load, steel yielding load and maximum load) unit: tonf

S p e c i m e n		A	B	C	
V e r t i c a l l o a d		100	500	100	
C o n c r e t e c r a c k i n g l o a d	Measured value P_{cm}	29.7	54.7	30.0	
	Theoretical value P_{cc}	32.2	47.5	29.7	
	P_{cm}/P_{cc}	0.92	1.15	1.01	
S t e e l y i e l d i n g l o a d	Measured value P_{ym}	114.6	74.7	109.8	
	Theoretical v a l u e	P_{tys}	108.3	73.0	98.3
		P_{tysc}	141.2	137.3	128.9
	P_{ym}/P_{tys}	1.06	1.02	1.12	
P_{ym}/P_{tysc}	0.81	0.54	0.85		
M a x i m u m l o a d	Measured value P_{mm}	192.6	194.5	184.6	
	Theoretical v a l u e	P_{uh}	169.4	168.9	150.1
		P_{up}	178.3	201.0	158.2
	P_{mm}/P_{uh}	1.14	1.15	1.22	
P_{mm}/P_{up}	1.08	0.97	1.17		

the measured steel yielding load(P_{ys}) is smaller than the value P_{tysc} and almost equal to the value P_{tyc}. On the other hand, the measured maximum loads(P_{mm}) exceed the value P_{uh} and almost agree with the value P_{up}. Thus, it is realized that partially-concrete-filled steel columns do not entirely behave as a composite structure in the elastic region, but do not lose its resistance abruptly after the steel yielding. Furthermore, it is understood that the load carrying capacity of partially-concrete-filled steel columns tends to become smaller by the effect of vertical load, because P_{mm}/P_{up}(=0.97) of Specimen-B is smaller than the ratio(=1.08) of Specimen-A and the ratio(=1.17) of Specimen-C.

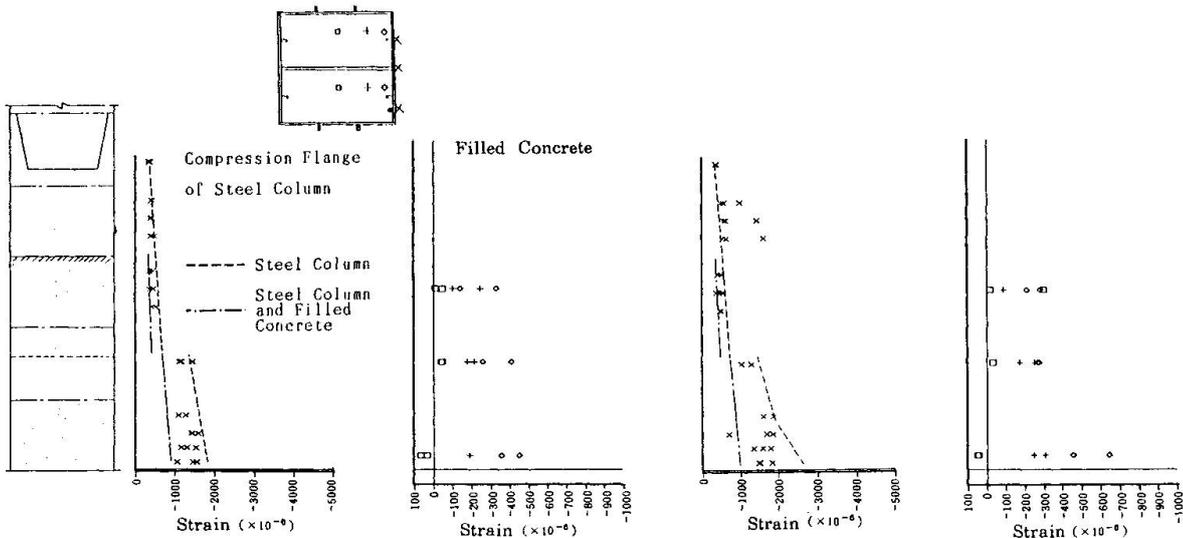
Table 4 Assumed stress distributions in cross section

Concrete cracking load	Steel yielding load		Maximum load	
	P _{cc}	P _{tyac}	P _{tya}	P _{uh}
Steel;	Steel;	Steel;	Steel;	Steel;
Concrete;	Concrete;	Concrete;	Concrete;	Concrete;

- σ_{ct}: Tensile strength of concrete
- σ_{sy}: Yield point of steel
- σ_{lb}: Local buckling strength of steel
- σ_{cc}: Compression strength of concrete

3.3 Characteristics of strain distribution

Fig. 4 (a) and (b) show the longitudinal strain distribution of the steel column and the filled concrete for Specimen-A and Specimen-C to understand the effect of the vertical diaphragm on the stress flow around the boundary between concrete-filled part and non



(a) Specimen-A (Lateral load: 114.5ton) (b) Specimen-C (Lateral load: 114.8ton)

Fig. 4 Longitudinal strain distribution



-concrete-filled part. In these figures, dashed lines are based on the assumption that only steel column resists the lateral load, while dot-dashed lines are based on the assumption that both steel column and the filled concrete resist the lateral load.

It is observed from these figures that longitudinal strains for Specimen-C exist widely around the boundary between concrete-filled part and non-concrete-filled part compared with that for Specimen-A. Thus, it is understood that the vertical diaphragm has the effect of making smooth stress flow around the boundary between concrete-filled part and non-concrete-filled part.

4. Concluding remarks

It was found that partially-concrete-filled steel columns do not exhibit substantial local buckling after the steel yielding, and have high load carrying capacity corresponding to almost full plastic resisting bending moment. It was also understood that the stress flow near the boundary between concrete-filled part and non-concrete-filled part become smooth by providing a vertical diaphragm.

[Reference]

- 1) Hirohiko Tada, Hiroshi Shinohara, Koichi Minosaku, Akira Takizawa and Jun Hikino; A Study on Behavior of Concrete Filled Steel Tubes, Proc. of the 2nd Bridge Workshop, 1985.
- 2) Guideline for Design and Construction of Steel Piers consisting of Composite Columns (Concrete Filled Type)-Draft-; Hanshin Expressway Public Corporation, 1986,3.