

**Zeitschrift:** IABSE reports of the working commissions = Rapports des commissions de travail AIPC = IVBH Berichte der Arbeitskommissionen  
**Band:** 6 (1970)  
**Artikel:** Creep effects in some arch and cantilever bridges  
**Autor:** Borges, J. Ferry / Marecos, J. / Trigo, J. Teixeira  
**DOI:** <https://doi.org/10.5169/seals-7768>

### **Nutzungsbedingungen**

Die ETH-Bibliothek ist die Anbieterin der digitalisierten Zeitschriften auf E-Periodica. Sie besitzt keine Urheberrechte an den Zeitschriften und ist nicht verantwortlich für deren Inhalte. Die Rechte liegen in der Regel bei den Herausgebern beziehungsweise den externen Rechteinhabern. Das Veröffentlichen von Bildern in Print- und Online-Publikationen sowie auf Social Media-Kanälen oder Webseiten ist nur mit vorheriger Genehmigung der Rechteinhaber erlaubt. [Mehr erfahren](#)

### **Conditions d'utilisation**

L'ETH Library est le fournisseur des revues numérisées. Elle ne détient aucun droit d'auteur sur les revues et n'est pas responsable de leur contenu. En règle générale, les droits sont détenus par les éditeurs ou les détenteurs de droits externes. La reproduction d'images dans des publications imprimées ou en ligne ainsi que sur des canaux de médias sociaux ou des sites web n'est autorisée qu'avec l'accord préalable des détenteurs des droits. [En savoir plus](#)

### **Terms of use**

The ETH Library is the provider of the digitised journals. It does not own any copyrights to the journals and is not responsible for their content. The rights usually lie with the publishers or the external rights holders. Publishing images in print and online publications, as well as on social media channels or websites, is only permitted with the prior consent of the rights holders. [Find out more](#)

**Download PDF:** 09.07.2025

**ETH-Bibliothek Zürich, E-Periodica, <https://www.e-periodica.ch>**

## **Creep Effects in some Arch and Cantilever Bridges**

Effets du fluage sur quelques ponts en arc et en encorbellement

Kriechwirkungen in einigen Bogen- und Freivorbaubrücken

**J. FERRY BORGES**

Associate Director

**J. MARECOS**

Head, Division of Observation of Structures

Laboratório Nacional de Engenharia Civil, Lisbon  
Portugal

**J. TEIXEIRA TRIGO**

Research Officer

### **1 – INTRODUCTION**

The techniques followed by the Laboratório Nacional de Engenharia Civil for the observation of structures, mainly bridges and dams, have been published at several instances (1 to 6). The present paper simply calls the attention to the techniques used for the measurement of strains and stresses, particularly taking into account the effects of creep.

For the accurate measurement of strains and stresses in reinforced and prestressed concrete structures acoustic strain gauges embedded in the concrete are used. These strain gauges have cylindrical body and heads and total lengths varying between 21 and 34 cm. They yield accuracies of the order of  $5 \times 10^{-6}$ . (4)

When measuring strains the main difficulty consists in distinguishing the part due to stresses from the part due to shrinkage and temperature. Also the evolution of the mechanical properties with time - creep effect - is in general difficult to follow and to interpret. In order to overcome this difficulty the strain gauges are embedded in the structure at three different conditions: i) active gauges, ii) compensating gauges and iii) controlled

gauges, fig. 1.

The active gauges measure the total strains in the structure. The compensating gauges are introduced inside double wall cylindrical boxes, built of thin copper sheet. Thus they are subtracted from the action of stresses. The controlled gauges are also inserted into similar double wall cylindrical boxes. However metallic cushions filled with oil placed at the top of the boxes allow to apply known stresses on the concrete cylinders where the strain gauges are embedded. By controlling the pressure of the oil in the cushion, mechanical tests of the concrete inside the structure, particularly creep tests, can be performed.

By the simultaneous use of an active and a controlled gauge it is even possible to perform a direct measurement of stresses, not involving the knowledge of the mechanical properties of the concrete. In fact, if the pressure of the oil in the cushion is varied in such a way that the strains indicated by both the active and the controlled gauges are permanently equal, the pressure applied corresponds without further corrections to the existing stresses.

In practice instead of varying the pressure on the controlled gauge it is preferable to apply a constant pressure, which corresponds to performing a creep test. This procedure, as compared with the usual technique of performing creep tests in the laboratory, has the great advantage that a very close similitude between the conditions around the active and the controlled gauge is automatically obtained. In this way, the creep corrections to be introduced in the active gauges can be very accurately computed. Thus the overall accuracy of the results is great, as shall be seen.

In order to reach this high accuracy, the thermal and hygrometric conditions around the active, the compensating, and the controlled gauges must be as similar as possible. Consequently, in the case of structures formed by walls, the cylindrical boxes indicated above are substituted by simple thermal and hygrometric insulations, as indicated in fig. 2.

## 2 – RESULTS OF TESTS

Figs. 3 to 5 show the results of creep tests performed in three bridges: two arch bridges and a prestressed cantilever bridge. These creep tests refer to controlled gauges and were run during periods varying from 500 to 2000 days.

The creep tests at Abreiro bridge were performed from 1956 to 1958, during 900 days, by applying a constant stress of  $45 \text{ kgf/cm}^2$ , fig. 3.

The creep tests at Arrabida bridge were performed in 1962 and 1963, during 500 days, by applying a constant stress of  $80 \text{ kgf/cm}^2$ , fig. 4. The measurement of strains and displacements is still being continued at the present date. However the creep tests were discontinued in 1963 due to rupture of the metallic cushions of the controlled gauges.

The creep tests at the north viaduct of the Tagus River bridge started in 1964 and are still in progress. The results presented refer to a period of about 2000 days. The stress applied is  $100 \text{ kgf/cm}^2$ , fig. 5.

Fig. 6 indicates the results obtained by a group of strain gauges at section S of one arch of Arrabida Bridge. As can be seen the creep test was performed at a stress near to the one due to the applied loads.

The diagrams of the evolution of strains, shrinkage and creep in the prestressed viaducts of the Tagus River bridge are presented in (6).

Table I indicates for the different bridges: the mean value of the resistance of the concrete at 28 days, obtained by testing 20 cm cubes; the instantaneous modulus of elasticity; the creep coefficient corresponding to 500 days,  $\varphi_e$ ; and the 500 days modulus of elasticity, obtained by dividing the instantaneous modulus of elasticity by  $1 + \varphi_e$ .

The creep coefficient relates the strains due to creep  $\epsilon_f$ , with the instantaneous ones due to the applied stress,  $\epsilon_0$ . Linear creep is thus assumed.

The number of compression tests performed, from which the indicated mean values were computed, varied from about 20 in the case of Abreiro bridge to several hundred in the case of Arrabida bridge.

It is interesting to compare the results presented in Table I with those that would be obtained by using the expressions recommended by FIP-CEB (7).

Assuming that the experimental modulus of elasticity,  $E_e$ , corresponds to the secant modulus,  $E_s$ , as defined by FIP-CEB, R12,222 and that this last one is 10% lower than the tangent modulus,  $E_t$ ; adopting expression

$$E_t = 21000 \sqrt{R'_b}$$

where  $R'_b$  is the resistance in cylinders expressed in  $\text{kgf/cm}^2$ ; and further taking this resistance as 0.85 of the resistance in cubes  $R'_{bc}$ ; it comes

$$E_s = 0.9 \times 21000 \sqrt{0.85 R'_{bc}} = 17500 \sqrt{R'_{bc}}$$

The values of  $E_e$ ,  $E_s$  and  $E_s/E_e$  are indicated in Table II.

For Arrabida and Tagus River bridges the computed values differ respectively plus and minus 8% from the experimental ones. In case of Abreiro bridge the computed value is 36% higher than the experimental one. It must be referred that in this case the modulus of elasticity considerably increased in time and reached the mean value of  $2.7 \times 10^5 \text{ kgf/cm}^2$  at the age of 300 days.

As regards creep, FIP-CEB Recommendations give the creep coefficient  $\psi_t$  by the product of 5 coefficients

$$\psi_t = k_c k_d k_b k_e k_t$$

which, respectively, consider

- $k_c$  - relative humidity of the air
- $k_d$  - age at loading
- $k_b$  - amount of cement and water/cement ratio
- $k_e$  - fictitious thickness
- $k_t$  - time and fictitious thickness.

For the cases under consideration these coefficients take the values indicated in Table II and lead to the values  $\psi_t$  also indicated in the same table. The agreement between computed and measured values is very good for Arrabida and Tagus River bridges and poor for Abreiro bridge. Attention is called to the fact that in this case the coefficient  $k_t$  (influence of

time) takes a very low value due to the great thickness of the arch, about 100 cm.

In all the cases, the stresses at different sections of the bridges were computed by using the results of the active gauges, duly compensated by the controlled gauges.

In Abreiro bridge, the accuracy with which it was possible to compute the stresses was fairly good. However, due to technique improvements this accuracy was considerably increased in Arrabida and Tagus River bridges.

For instance, in the case of Arrabida bridge the axial forces computed from the measured stresses satisfy the conditions of equilibrium with a mean error of 2% and maximum errors of about 4%. In this bridge a total of 214 gauges were used to determine stresses at 8 sections. It is interesting to note that due to creep there was an important evolution of the bending moments in the sections of the arches. The evolution in time of the displacements agrees with the measured evolution of strains.

As is well known, creep effects are very important in cantilever prestressed concrete bridges. The measurements performed at the Tagus River bridge allowed an understanding of the general behaviour of the structure and contributed to an accurate forecasting of the evolution of the cantilever displacements in time.

### 3 – CONCLUSIONS

For studying the behaviour of reinforced and prestressed concrete structures accurate measurement of strains and stresses is needed. The measurement of stresses is particularly difficult due to creep effects.

Thus, for the observation of structures, the simultaneous use of active, compensating, and controlled gauges is advisable, the last ones allowing to perform compression tests, particularly creep tests, in the structure, in just the same conditions as those under which the other gauges are operating.

The results on creep obtained at Arrabida arch bridge and at the prestressed cantilever viaduct of Tagus River bridge agree closely with the creep expression presented at the "FIP-CEB Recommendations". For Abreiro

bridge the agreement is much worse.

In these three bridges the measurement of stresses was performed along several years with high accuracy. The verification of equilibrium conditions allowed to check this accuracy.

### References

- 1 - ROCHA, M.; BORGES, J.FERRY; MARECOS, J. - Observation of some reinforced concrete structures - RILEM Symposium on the Observation of Structures, Lisbon, 1955.
- 2 - ROCHA, M.; BORGES, J.FERRY; SERAFIM, J.L. - General report, RILEM Symposium on the Observation of Structures, Lisbon, 1955.
- 3 - ROCHA, M.; SILVEIRA, A.F. - Assessment of observation techniques used in Portuguese concrete dams, 8th Congress on Large Dams, Edinburgh, 1964.
- 4 - ROCHA, M. - In situ strain and stress measurements, paper included in "Recent developments in numerical and experimental methods" edited by Zienkiewicz - Holister, Swansea, 1965.
- 5 - BORGES, J.FERRY and MARECOS, J. - Bridge Observations in Portugal, Proceedings of the Conference on Experimental Methods of Investigating Stress and Strain in Structures, Praha, October, 1965.
- 6 - MARECOS, J.; CASTANHETA, M. and TRIGO, J.T. - Field observation of Tagus River suspension bridge, Proceedings of the American Society Civil Engineers, ST4, April, 1969.
- 7 - COMITÉ EUROPÉEN DU BÉTON - FÉDÉRATION INTERNATIONALE DE LA PRÉCONTRAÎNTE, Recommandations Internationales pour le Calcul et l'Exécution des Ouvrages en Béton, Seconde Edition, Juin, 1970.

TABLE I

BRIDGE	28 DAYS MEAN RESISTANCE $R_{bc}$ (kgf/cm <sup>2</sup> )	INSTANTANEOUS MODULUS OF ELASTICITY $E_e$ (kgf/cm <sup>2</sup> )	500 DAYS CREEP COEFFICIENT $\varphi_e$ —	500 DAYS MODULUS OF ELASTICITY $E_{500}$ (kgf/cm <sup>2</sup> )
ABREIRO	3.0 10 <sup>2</sup>	2.2 10 <sup>5</sup>	0.80	1.2 10 <sup>5</sup>
ARRABIDA	5.6 10 <sup>2</sup>	3.8 10 <sup>5</sup>	0.75	2.2 10 <sup>5</sup>
TAGUS RIVER	3.9 10 <sup>2</sup>	3.7 10 <sup>5</sup>	1.72	1.4 10 <sup>5</sup>



TABLE II

	MODULUS OF ELASTICITY			CREEP COEFFICIENT		
	MEASURED $E_e$ (kgf/cm <sup>2</sup> )	COMPUTED $E_s$ (kgf/cm <sup>2</sup> )	$\frac{\text{COMPUTED}}{\text{MEASURED}}$ $E_s/E_e$	MEASURED $\varphi_e$	COMPUTED $k_c k_d k_b k_e k_t = \varphi_t$	$\frac{\text{COMPUTED}}{\text{MEASURED}}$ $\varphi_t/\varphi_e$
BRIDGE			—	—	—	—
ABREIRO	2.2 10 <sup>5</sup>	3.0 10 <sup>5</sup>	1.36	0.80	2.3 × 0.8 × 1.0 × 0.7 × 0.2 = 0.3	0.4
ARRABIDA	3.8 10 <sup>5</sup>	4.1 10 <sup>5</sup>	1.08	0.75	2.1 × 0.7 × 0.9 × 0.75 × 0.7 = 0.7	0.93
TAGUS RIVER	3.7 10 <sup>5</sup>	3.4 10 <sup>5</sup>	0.92	1.72	2.3 × 1.0 × 1.4 × 0.75 × 0.7 = 1.7	0.98

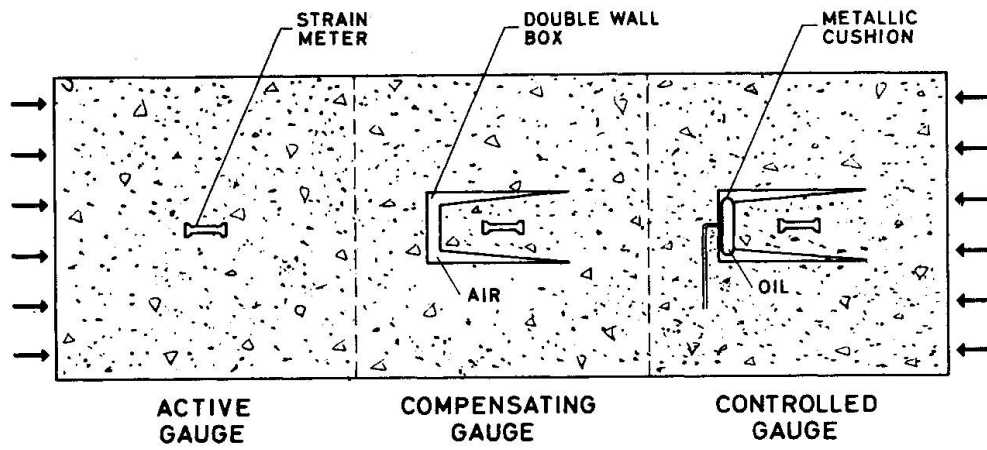


Fig. 1

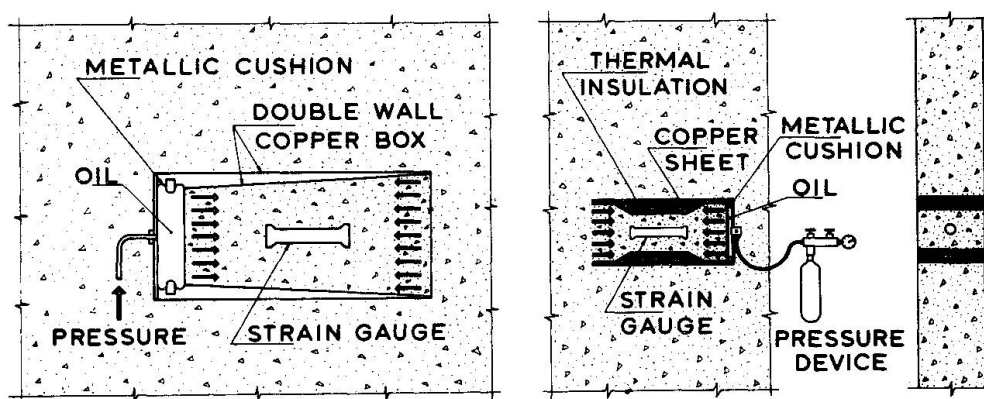


Fig. 2

## ABREIRO BRIDGE

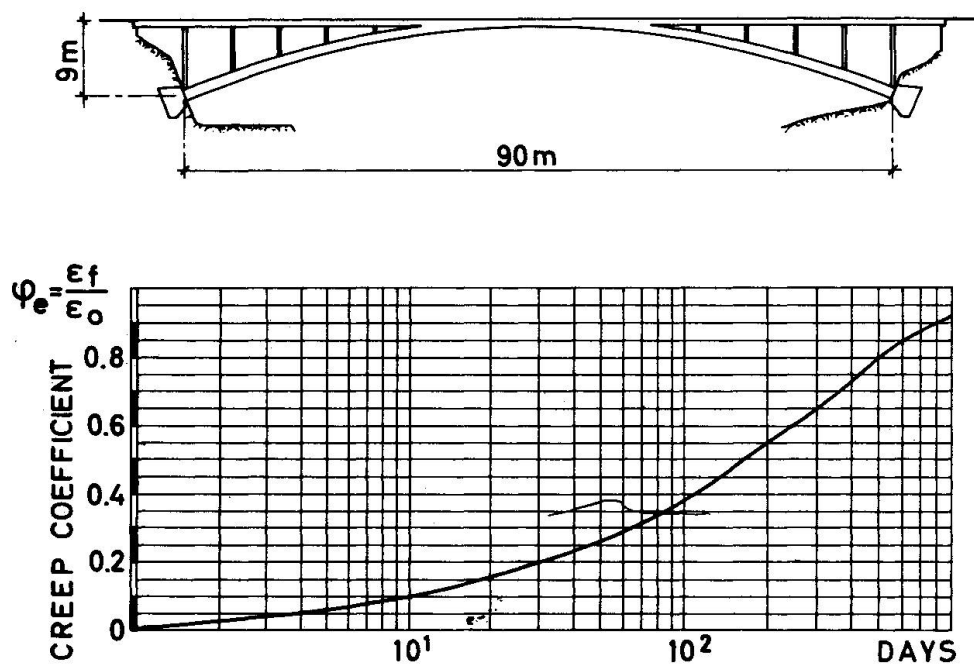


Fig. 3

## ARRABIDA BRIDGE

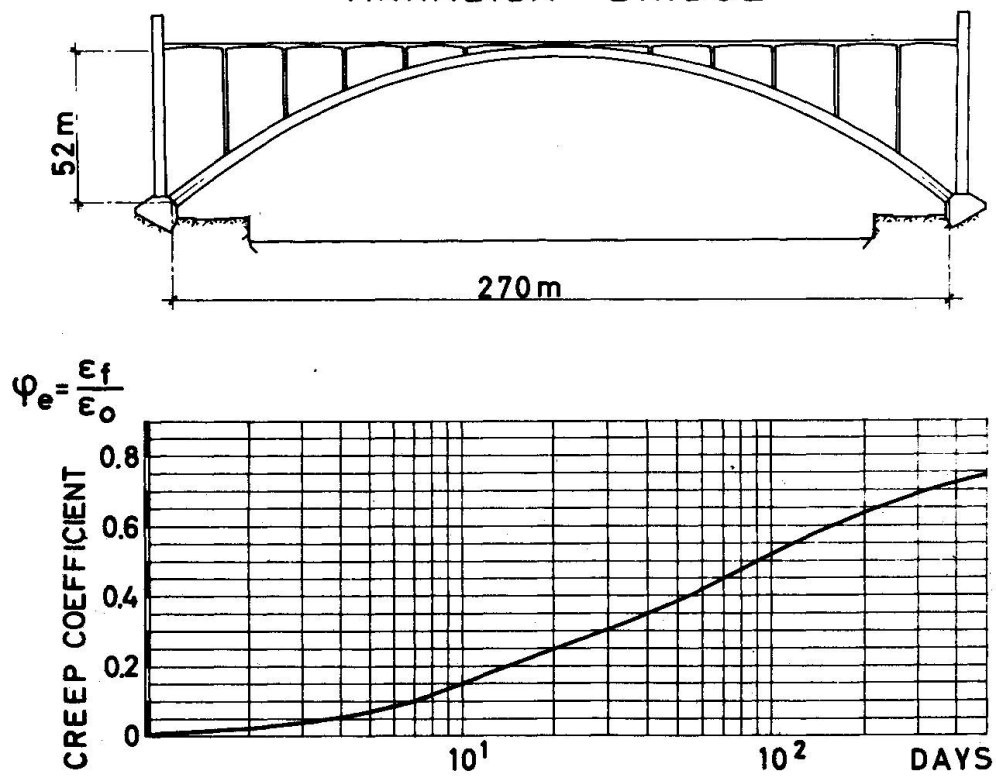


Fig. 4

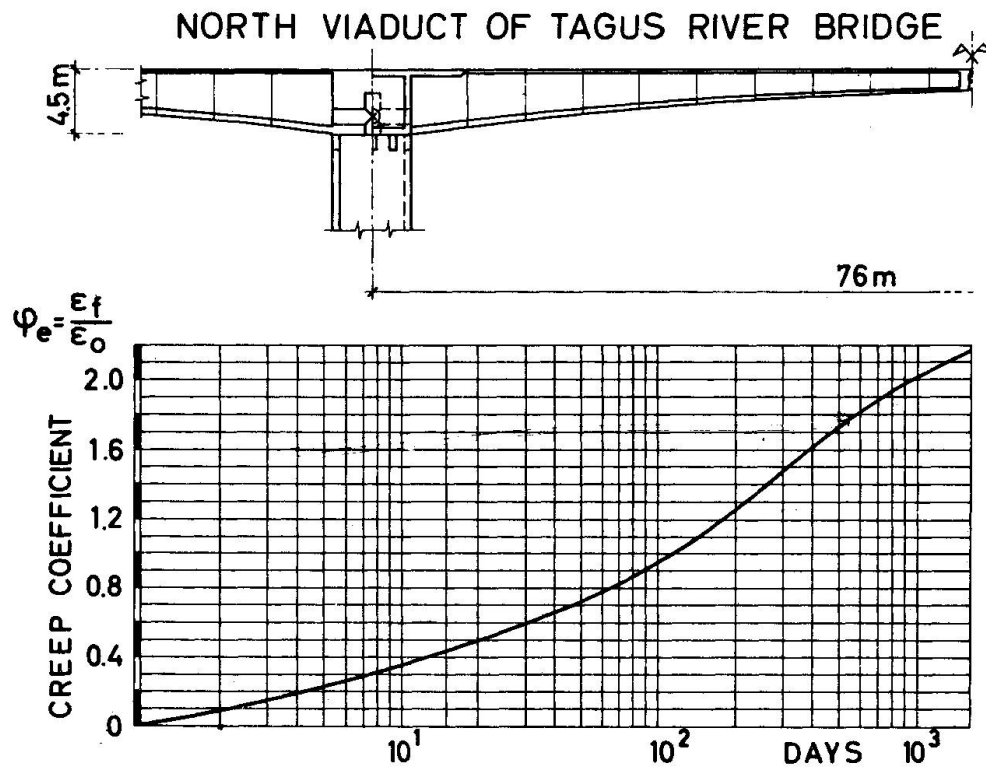


Fig. 5

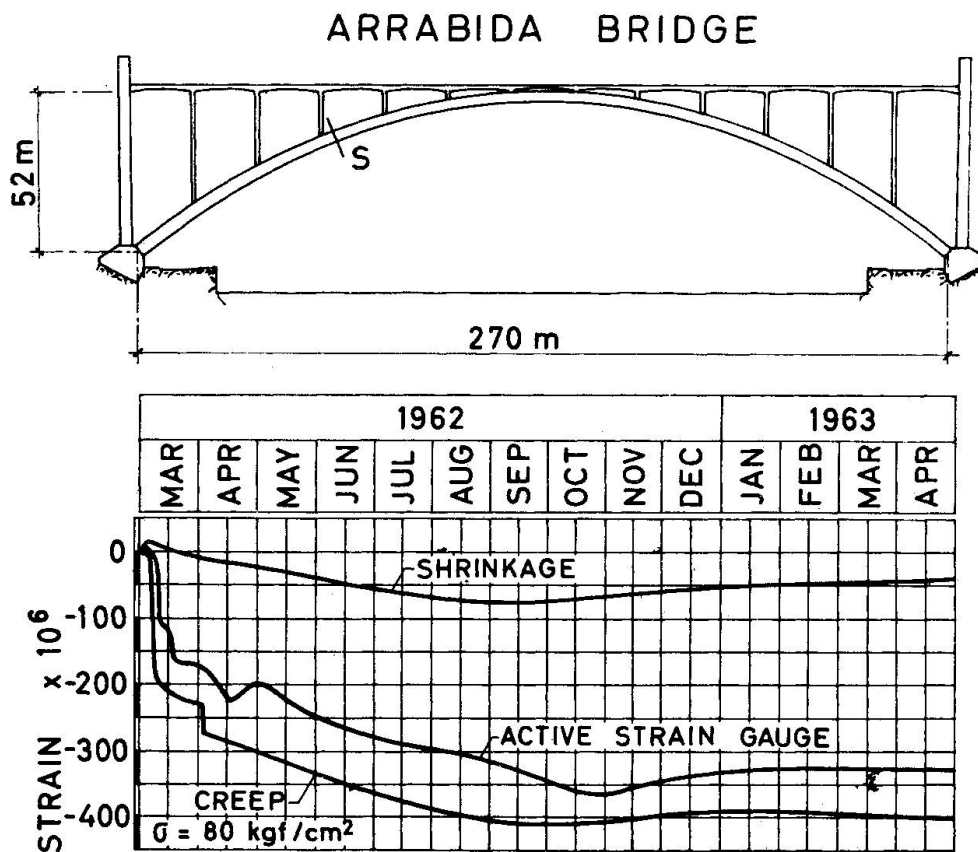


Fig. 6

## SUMMARY

The techniques followed at LNEC, Lisbon, for the in situ measurement of strains and stresses in reinforced concrete structures are briefly described.

The results of creep tests performed in three bridges are presented and interpreted.

Attention is called to the advantages of performing creep tests in the bridges using controlled gauges.

## RESUME

On décrit sommairement les techniques suivies au LNEC pour mesurer in situ les déformations et les contraintes des ouvrages en béton armé.

Les résultats des essais de fluage effectués sur trois ponts sont présentés et interprétés.

On souligne les avantages d'effectuer des essais de fluage à l'aide d'extensomètres contrôlés.

## ZUSAMMENFASSUNG

Es werden die Verformungs- und Spannungsmessungstechniken kurz beschrieben, die im LNEC, Lissabon, für Ortsmessungen angewandt werden.

Die an drei Brücken durchgeführten Kriechversuche werden dargestellt und ausgewertet.

Es werden auch die Vorteile hervorgehoben, die sich aus mit kontrollierten Dehnungsmessern durchgeführten Kriechversuchen ergeben.